		REQUIRI	ED? (Y/N)	MATERIAL / ACTIVITY
CODE: INTERNATIONAL BUILDING CODE (IBC)	2018	Y	Ν	1704.2.5 Inspection of Fabricators Verify fabrication/quality control procedures
LOADINGS FLOOR LIVE LOAD	40 PSF	Y	Ν	1705.1.1 Special Cases (work unusual in nature, including but not limited to alter
DECK LIVE LOAD ROOF SNOW LOAD	60 PSF			and systems, unusual design applications, materials and systems with special m requirements)
WIND CRITERIA BUILDING CLASSIFICATION	11			1705.2 Steel Construction
ULTIMATE WIND SPEED WIND EXPOSURE	" 110 MPH B	Y	N	 Fabricator and erector documents (Verify reports and certificates as listed in A N, paragraph 3.2 for compliance with construction documents)
TOPOGRAPHIC FACTOR, Kzt	1.6	Y Y	N N	 Material verification of structural steel Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 fc
SEISMIC CRITERIA SEISMIC RISK CATEGORY	II	Y	Ν	4. Verify member locations, braces, stiffeners, and application of joint details at e comply with construction documents
SPECTRAL RESPONSE COEFFICIENT, Ss SPECTRAL RESPONSE COEFFICIENT, S1	1.33 0.50	Y	Ν	5. Structural steel welding: a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint o
SEISMIC SITE CLASS SEISMIC DESIGN CATEGORY	D D	Y	Ν	tasks listed in AISC 360, Table N5.4-1) b. Inspection tasks During Welding (Observe, or perform for each welded joint or tasks listed in AISC 360, Table N5.4-1)
STRUCTURAL DESCRIPTIONS		Y	Ν	c. Inspection tasks After Welding (Observe, or perform for each welded joint or n tasks listed in AISC 360, Table N5.4-3)
		Y	N	 d. Nondestructive testing (NDT) of welded joints: see Commentary 1) Complete penetration groove welds 5/16" or greater in risk category III or IV
		Y Y	N N	 2) Complete penetration groove welds 5/16" or greater in risk category II 3) Thermally cut surfaces of access holes when material t > 2"
GENERAL CONDITIONS		Y Y	N N	4) Welded joints subject to fatigue when required by AISC 360, Appendix 3, Tabl5) Fabricator's NDT reports when fabricator performs NDT
1. THE CONTRACTOR SHALL EXAMINE THE STRUC STRUCTURAL ENGINEER IN WRITING OF ANY DISCREPA		Y	Ν	6. Structural steel bolting:a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted co
THE WORK. THE CONTRACTOR SHALL VERIFY ALL DIM STARTING WORK.		E _Y	Ν	accordance with QA tasks listed in AISC 360, Table N5.6-1) b.Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Tab
	VARIOUS ELEMENTS OF THE WORKING DRAWINGS	Y Y	N	 Pre-tensioned and slip-critical joints Turn-of-nut with matching markings Direct tension in directory
SHALL BE BROUGHT TO THE ATTENTION OF THE ARCH PROCEEDING WITH ANY WORK SO INVOLVED.		Y Y V	N N N	 b) Direct tension indicator c) Twist-off type tension control bolt d) Ture of put without matching markings
3. SPECIFIC NOTES AND DETAILS SHALL TAKE PRI	ECEDENCE OVER GENERAL NOTES AND TYPICAL	Y	N	d) Turn-of-nut without matching markings e) Calibrated wrench 2) Snug-tight joints
DETAILS. WHERE THE NOTES, DRAWINGS, AND/OR SPI REQUIREMENT SHALL APPLY.	ECIFICATIONS DIFFER, THE MORE STRINGENT	Y	Ν	 c. Inspection tasks After Bolting (Perform tasks for each bolted connection in acc tasks listed in AISC 360, Table N5.6-3)
	PART OF THE WORK, THE CONSTRUCTION SHALL BE	Y	Ν	 7. Inspection of steel elements of composite construction prior to concrete placer with QA tasks listed in AISC 360, Table N6.1
THE SAME AS FOR SIMILAR WORK.				1705.2.2 Steel Construction Other Than Structural Steel
5. WORKING DIMENSIONS SHALL NOT BE SCALED DRAWINGS.	FROM PLANS, SECTIONS, OR DETAILS ON THESE	Y	N	1. Material verification of cold-formed steel deck: a. Identification markings
	(THE ARCHITECT AND THE STRUCTURAL ENGINEER	Y	Ν	 b. Manufacturer's certified test reports 2. Connection of cold-formed steel deck to supporting structure:
OF ANY CONDITION THAT, IN HIS OPINION, MIGHT ENDA DISTRESS TO THE STRUCTURE.	NGER THE STABILITY OF THE STRUCTURE OR CAUSE	Y Y	N N N	a. Welding b. Other fasteners (in accordance with AISC 360,Section N6)
7. THE CONTRACTOR SHALL SUPERVISE AND DIR RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS		Y Y	N N	 Verify fasteners are in conformance with approved submittal Verify fastener installation is in conformance with approved submittal and mar
PROVIDE ADEQUATE SHORING AND BRACING OF ALL S NOTIFY ENGINEER OF ALL FIELD CHANGES PRIOR TO IN	TRUCTURAL MEMBERS DURING CONSTRUCTION.		N	recommendations 3. Reinforcing steel
	R INFORMATION NOT COVERED BY THESE GENERAL	Y Y	N N	a. Verification of weldability of steel other than ASTM A706b. Reinforcing steel resisting flexural and axial forces in intermediate and special
NOTES OR THE STRUCTURAL DRAWINGS.		Y Y	N N	boundary elements of special concrete structural walls and shear reinforcement c. Shear reinforcement
 ALL CONSTRUCTION SHALL BE DONE WITH MAT AS GOOD PRACTICE BY THE CONSTRUCTION INDUSTR' PREVAILING CODE EDITION OF THE "INTERNATIONAL BI THEREIN. 		Ŷ	N	d. Other reinforcing steel 4. Cold-formed steel trusses spanning 60 feet or greater a. Verify temporary and permanent restraint/bracing are installed in accordance v truss submittal package
	CHASES, BLOCK-OUTS, ETC., SHALL NOT BE PLACED	Y	N	1705.3 Concrete Construction Inspection of reinforcing steel installation (see 1705.2.2 for welding)
IN SLABS, FOUNDATIONS, ETC., NOR SHALL ANY STRUC SPECIFICALLY DETAILED ON THESE STRUCTURAL DRA		Ý Y	N N	 Inspection of prestressing steel installation Inspection of prestressing steel installation Inspection of anchors cast in concrete where allowable loads have been increation
	BE CONSIDERED FOR REVIEW. ENGINEER MAY	(\mathbf{Y})	N	1908.5 or where strength design is used4. Inspection of anchors and reinforcing steel post-installed in hardened concrete
REQUEST PAYMENT FOR REVIEW.		\bigcirc		reports including verification of anchor type, anchor dimensions, hole dimensions procedures, anchor spacing, edge distances, concrete minimum thickness, anch
FOUNDATION 1. STRUCTURAL DESIGN COMPLIES WITH SOILS R		Y	Ν	tightening torque 5. Verify use of approved design mix
N.A.		Y	N	Fresh concrete sampling, perform slump and air content tests and determine t concrete
FOOTING BEARING PRESSURE:	1500 PSF (ASSUMED)	Y Y	N N	 Inspection of concrete and shotcrete placement for proper application techniques Inspection for maintenance of specified curing temperature and techniques
LATERAL EARTH PRESSURE ON RETAINING WA	LLS N.A.	Y Y	N N	9. Inspection of prestressed concrete:a. Application of prestressing force
2. SUBGRADE PREPARATION, DRAINAGE PROVISION CONSIDERATIONS ARE TO BE IN ACCORDANCE		Y	N	 b. Grouting of bonded prestressing tendons in the seismic-force-resisting system 10. Erection of precast concrete members a. Inspect in accordance with construction decuments
		Y Y	N N	 a. Inspect in accordance with construction documents b. Perform inspections of welding and bolting in accordance with Section 1705.2 11. Verification of in-situ concrete strength, prior to stressing of tendons in post to
		Ŷ	N	and prior to removal of shores and forms from beams and structural slabs 12. Inspection of formwork for shape, lines, location and dimensions
		Y	Ν	13. Concrete strength testing and verification of compliance with construction do
DIMENSIONAL LUMBER, ANCHOR BOLT AND NAILING SF	PECIFICATIONS			Notes:
1. MEET REQUIREMENTS OF PS 20-70 AND NATIONAL G				1. The inspection and testing agent(s) shall be engaged by the Owner or the Own not by the Contractor or Subcontractor whose work is to be inspected or tested.
DIMENSIONAL LUMBER. BEAR STAMP OF WWPA.				interest must be disclosed to the Building Official prior to commencing work. The the Special Inspector(s) and/or testing agencies may be subject to the approval of the special loss of t
2. MINIMUM DIMENSIONAL LUMBER GRADES TO BE:				Official and/or the Design Professional. 2. The list of Special Inspectors may be submitted as a separate document, if no
WALL STUDS, 2X, 3 X HF STUD GRADE WALL PLATES, 2X, 3X HF STANDARD G				2. Special Inconstigue of required by Section 1704.2.5 are not required where the
JOISTS, 2 X 6: HF #2 JOISTS, 2 X 8 AND UP DF #2				3. Special Insepctions as required by Section 1704.2.5 are not required where the approved in accordance with IBC Section 1704.2.5.2
BEAMS, HEADERS, 6X DF #2 BEAMS, HEADERS, 4X DF #2, WWPA GR	ADING			Observe on a random basis, operations need not be delayed pending these in these tasks for each welded joint, bolted connection, or steel element.
POSTS, 4X, 6X DF #2 U.N.O LUMBER NOT NOTED HERE DF #2 U.N.O				5. NDT of welds completed in an approved fabricator's shop may be performed b
3. PROVIDE STANDARD CUT WASHERS FOR BOLT HEAD WOOD.	OS AND NUTS BEARING AGAINST			when approved by the AHJ. Refer to AISC 360, N7.
4. ALL SILLS OR PLATES RESTING ON CONCRETE OR M	ASONRY THAT IS IN CONTACT WITH C	ONCRETE	AND REIN	FORCING
OR RESTING ON FOUNDATIONS SHALL BE PRESSURE-T ACCORDANCE TO WITH AWPA U1 (PLANT/SHOP TREAT	MENT) AND M4 (FIELD TREATMENT) 1.			HALL CONFORM TO THE INDICATED REFERENCE CODES AND STANDARDS
STANDARDS. ALL BEARING WALL PLATES SHALL HAVE MAXIMUM OF 9" FROM THE END OF A PLATE AND SPAC	ED AT INTERVALS SHOWN ON THE			DDIFIED BELOW:
SHEARWALL SCHEDULE (MAXIMUM 4'-0" OC SPACING). FOUNDATION SILL PLATE ANCHOR BOLTS. PROVIDE TV SECTION OF SILL. FOR NON-SHEARWALL, PLACE ANCH	VO ANCHOR BOLTS MINIMUM PER	ACI-	318 - "BUIL	LDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE TWEATHER CONCRETING"
5. BOLTS IN WOOD SHALL NOT BE LESS THAN 7 DIAMET		ACI-	306R - "CO	D WEATHER CONCRETING DE FOR MEASURING, MIXING, TRANSPORTING AND PLACING CONCRETE
DIAMETERS FROM THE EDGE OF THE MEMBER.	2.			X SPECIFICATIONS
6. NAILS: COMMON WIRE NAILS. NAILING IN ACCORDAI				
7. PRESSURE TREATED WOOD: ALL NAILS INTO PT WO GALVANIZED PER ASTM A153 OR STAINLESS STEEL. AL			ATION	COMP. SRENGTH W/C RATIO AIR CONTENT REMARK
WITH PT WOOD SHALL BE HOT DIPPED GALVANIZED AN OZ OF ZINC PER SQ FT MINIMUM) OR TYPE 304 / 316 ST/	ID MEET ASTM A653 CLASS G185 (1.85 AINLESS STEEL SIMPSON Z-MAX			2500 PSI (MIN. OF 5.5 SACKS OF CEMENT PER CUBIC YARD OF CON
CONNECTORS MEET THIS REQUIREMENT. FASTENERS TOGETHER SHALL BE OF THE SAME TYPE (E.G. HOT DIR	AND CONNECTORS USED		B ON GRAD	
HANGERS)		FOU	INDATION \	WALL 2500 PSI (MIN. OF 5.5 SACKS OF CEMENT PER CUBIC YARD OF CON

8. ALL LUMBER WITH A LEAST DIMENSION OF 2" (NOMINAL) SHALL BE STAMPED "SURFACE-DRY" AND SHALL HAVE A MOISTURE CONTENT WHEN SURFACED AND WHEN INSTALLED OF NO MORE THAN 19 PERCENT. LUMBER WITH A LEAST DIMENSION OF 4" (NOMINAL) OR GREATER SHALL BE STAMPED "SURFACE-GREEN" AND AIR-DRIED TO A MOISTURE CONTENT OF NOT MORE THAN 19 PERCENT PRIOR TO ITS USE IN FRAMING THE

9. NOTCHING AND BORING OF BEAMS AND JOISTS IS NOT ALLOWED WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.

STRUCTURE.

a. TOTAL AIR CONTENT IS SPECIFIED IN THE TABLE ABOVE. AIR CONTENT TOLERANCE SHALL BE ± 1% AND SHALL BE MEASURED AT THE POINT OF PLACEMENT. (AFTER PUMPING IF APPLICABLE). ALL CONCRETE EXPOSED TO THE WEATHER SHALL HAVE AN APPROVED ADMIXTURE TO ENTRAIN AIR - 5% TOTAL AIR REQUIRED. CONCRETE THAT CAN BE SUBJECTED TO FREEZING AND THAWING DURING CONSTRUCTION SHALL BE AIR ENTRAINED.

3. PROVIDE GRADE 60 KSI (A615) FOR CONCRETE STEEL REINFORCING

N.A.

TOPPING

	EXTENT	REQUIRE	D? (Y/N)	MATERIAL / ACTIVITY
	Periodic			1705.4 Masonry Construction
		Y	N	(A) Level A, B and C Quality Assurance: 1. Verify compliance with approved submittals
re, including but not limited to alternative materials terials and systems with special manufacturer's		Y	N	(B) Level B Quality Assurance: 1. Verification of f'm and f'AAC prior to construction
		Y	Ν	(C) Level C Quality Assurance: 1. Verification of f'm and f'AAC prior to construction and for every 5,000 SF during co
		Ŷ	N	 Verification of proportions of materials in premixed or preblended mortar, prestress grout other than self-consolidating grout, as delivered to the project site
ports and certificates as listed in AISC 360, chapter tion documents)	Each submittal	Y	N	3. Verify placement of masonry units (D) Levels B and C Quality Assurance:
length, embedment. See 1705.3 for anchors)	Periodic Continuous	Y	Ν	 Verification of Slump Flow and Visual Stability Index (VSI) of self-consolidating grout the project
and application of joint details at each connection	Periodic	Y Y	N N	 Verify compliance with approved submittals Verify proportions of site-mixed mortar, grout and prestressing grout for bonded ter
or perform for each welded joint or member, the QA	Observe or Perform as noted (4)	Y	Ν	 Verify grade, type, and size of reinforcement and anchor bolts, and prestressing ter anchorages
or perform for each welded joint or member, the QA	Observe (4)	Y Y	N N	 Verify construction of mortar joints Verify placement of reinforcement, connectors, and prestressing tendons and anch
perform for each welded joint or member, the QA	Observe or Perform as noted (4)	Y	N	7. Verify grout space prior to grouting
ts: see Commentary		v	N	8. Verify placement of grout and prestressing grout for bonded tendons
greater in risk category III or IV	Periodic Periodic	Ý Y	N N	 9. Verify size and location of structural masonry elements 10. Verify type, size, and location of anchors, including details of anchorage of mason
r greater in risk category II n material t > 2"	Periodic			members, frames, or other construction.
ed by AISC 360, Appendix 3, Table A-3.1 rforms NDT	Periodic Each submittal (5)	Y Y	N N	 Verify welding of reinforcement (see 1705.2.2) Verify preparation, construction, and protestion of masonry during cold weather (to a second seco
r perform tasks for each bolted connection, in	Observe or Perform as noted (4)	Y	N	40oF) or hot weather (temperature above 90oF) 13. Verify application and measurement of prestressing force
Table N5.6-1) QA tasks listed in AISC 360, Table N5.6-2)	Observe (4)	Y	Ν	14. Verify placement of AAC masonry units and construction of thin-bed mortar joints AAC masonry)
	Periodic	Y	Ν	15. Verify placement of AAC masonry units and construction of thin-bed mortar joints 5000 SF of AAC masonry)
	Periodic Periodic	Y Y	N N	16. Verify properties of thin-bed mortar for AAC masonry (first 5000 SF of AAC maso 17. Verify properties of thin-bed mortar forAAC masonry (after the first 5000 SF of AA
	Continuous Continuous	Y	N	18. Prepare grout and mortar specimens
	Periodic	-		
s for each bolted connection in accordance with QA	Perform (4)	Y	Ν	19. Observe preparation of prisms
onstruction prior to concrete placement in accordance	Observe or Perform as noted (4)	Y	Ν	1705.5 Wood Construction1. Inspection of the fabrication process of wood structural elements and assemblies in
uctural Steel		Y	N	Section 1704.2.5 2. For high-load diaphragms, verify grade and thickness of structural panel sheathing
sk:	Periodic	Y	N	approved building plans 3. For high-load diaphragms, verify nominal size of framing members at adjoining par
porting structure:	Each submittal			staple diameter and length, number of fastener lines, and that spacing between faster and at edge margins agree with approved building plans
	Periodic	Y	Ν	 4. Metal-plate-connected wood trusses spanning 60 feet or greater: verify temporary a restraint/bracing are installed in accordance with the approved truss submittal package
60,Section N6) proved submittal	Periodic			
e with approved submittal and manufacturer's	Periodic			1705.6 Soils1. Verify materials below shallow foundations are adequate to achieve the design bea
ASTM A706	Periodic	Y Y	N N	 Verify excavations are extended to proper depth and have reached proper material Perform classification and testing of controlled fill materials.
forces in intermediate and special moment frames, ral walls and shear reinforcement	Continuous	Y Y	N N	Verify use of proper materials, densities, and lift thicknesses during placement and controlled fill
	Continuous Periodic	Y	N	5. Prior to placement of controlled fill, observe subgrade and verify that site has been
or greater acing are installed in accordance with the approved	Periodic			1705.7 Driven Deep Foundations Verify element materials, sizes and lengths comply with requirements
		Y Y	N N	 Determine capacities of test elements and conduct additional load tests, as require Observe driving operations and maintain complete and accurate records for each elements
e 1705.2.2 for welding)	Periodic.	Ý Y	N N	 Verify placement locations and plumbness, confirm type and size of hammer, record per foot of penetration, determine required penetrations to achieve design capacity, re
	Periodic Continuous	·		elevations and document any damage to foundation element 5. For steel elements, perform additional inspections per Section 1705.2
e allowable loads have been increased per section	Periodic or as required by the research report issued by an	Y Y	N N	6. For concrete elements and concrete-filled elements, perform additional inspections
oost-installed in hardened concrete: Per research ichor dimensions, hole dimensions, hole cleaning	approved source			1705.37. For specialty elements, perform additional inspections as determined by the register
concrete minimum thickness, anchor embedment and		Y	N	professional in responsible charge 8. Perform additional inspections and tests in accordance with the construction docur
d air content tests and determine temperature of	Periodic Continuous	Y	Ν	1705.8 Cast-in-Place Deep Foundations
ent for proper application techniques	Continuous	Y	N	1.Observe drilling operations and maintain complete and accurate records for each el 2. Verify placement locations and plumbness, confirm element diameters, bell diamet
ng temperature and techniques	Periodic	Y	Ν	lengths, embedment into bedrock (if applicable) and adequate end-bearing strata cap concrete or grout volumes
the seismic-force-resisting system	Continuous Continuous	Y	Ν	 For concrete elements, perform additional inspections in accordance with Section ' Perform additional inspections and tests in accordance with the construction docur
uments	In accordance with construction documents	Ŷ	N	1705.9 Helical Pile Foundations
n accordance with Section 1705.2	In accordance with Section 1705.2 Periodic	Y	Ν	1. Verify installation equipment, pile dimensions, tip elevations, final depth, final instal
or to stressing of tendons in post tensioned concrete beams and structural slabs	Periodic	Ý		other data as required. 2. Perform additional inspections and tests in accordance with the construction docur
ation and dimensions f compliance with construction documents	Periodic	T	Ν	1705.10.1 Structural Wood Special Inspections For Wind Resistance
		Y	N	 Inspection of field gluing operations of elements of the main windforce-resisting sys Inspection of nailing, bolting, anchoring and other fastening of components within the
engaged by the Owner or the Owner's Agent, and		Y	Ν	resisting system
work is to be inspected or tested. Any conflict of ial prior to commencing work. The qualifications of				1705.10.2 Cold-formed Steel Special Inspections For Wind Resistance 1.Inspection during welding operations of elements of the main windforce-resisting sy
s may be subject to the approval of the Building		Y Y	N N	 Inspections for screw attachment, bolting, anchoring and other fastening of compon main windforce-resisting system
tted as a separate document, if noted so above.				
1704.2.5 are not required where the fabricator is		Y	N	1705.10.3 Wind-resisting Components 1. Roof cladding
1704.2.5 are not required where the fabricator is		Ý	N	2. Wall cladding
d not be delayed pending these inspections. Perform		Y	Ν	1705.11.1 Structural Steel Special Inspections for Seismic Resistance Inspection of structural steel in accordance with AISC 341
ection, or steel element.				1705.11.2 Structural Wood Special Inspections for Seismic Resistance
ricator's shop may be performed by that fabricator), N7.		Y	Ν	 Inspection of field gluing operations of elements of the seismic-force resisting system Inspection of nailing, bolting, anchoring and other fastening of components within the
		Y	Ν	resisting system
				1705.11.3 Cold-formed Steel Light-Frame Construction Special Inspections for S
NCE CODES AND STANDARDS		Y	N	Resistance 1. Inspection during welding operations of elements of the seismic-force-resisting sys
CONCRETE"		Ý	N	Inspections for screw attachment, bolting, anchoring and other fastening of compor seismic-force-resisting system
IRAL CONCRETE"	STRUCTURAL AND MISCELLANEOUS STEEL			
		SHALL BF HO	OT DIPPFN	GALVANIZED OR EPOXY PAINTED PER ARCHITECT

STEEL MEMBERS, HARDWARE, FASTENERS SHALL BE HOT DIPPED GALVANIZED OR EPOXY PAINTED PER ARCHITECT PAINT PER ASTM A780. COLOR TO MATCH EXISTING.

ENT PER CUBIC YARD OF CONCRETE)

ENT PER CUBIC YARD OF CONCRETE)

ENT PER CUBIC YARD OF CONCRETE)

REQUIREMENTS. ALL CUT, REPAIRED AND EXPOSED SURFACE SHALL BE PAINTED WITH (2) COAT OF 95% ZINC RICH

STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS: ASTM A500, GRADE B (Fy = 46 KSI) TUBE COLUMNS: WIDE FLANGE COLUMNS / BEAMS: ASTM 572 GR50 STEEL PIPE: ALL OTHER STEEL: BOLTS: BOLTS: ANCHOR BOLTS:

ANCHOR BOLTS:

SCHEDULE 40, CONFORMING TO ASTM A53, TYPE E OR S, GRADE B (Fy = 35 KSI.) ASTM A36 (Fy = 36 KSI) OR ASTM A992 ASTM A307 (WOOD/STEEL CONN) ASTM A325/A490 WITH LOCK WASHERS (STEEL/STEEL AND STEEL/CONC CONN)

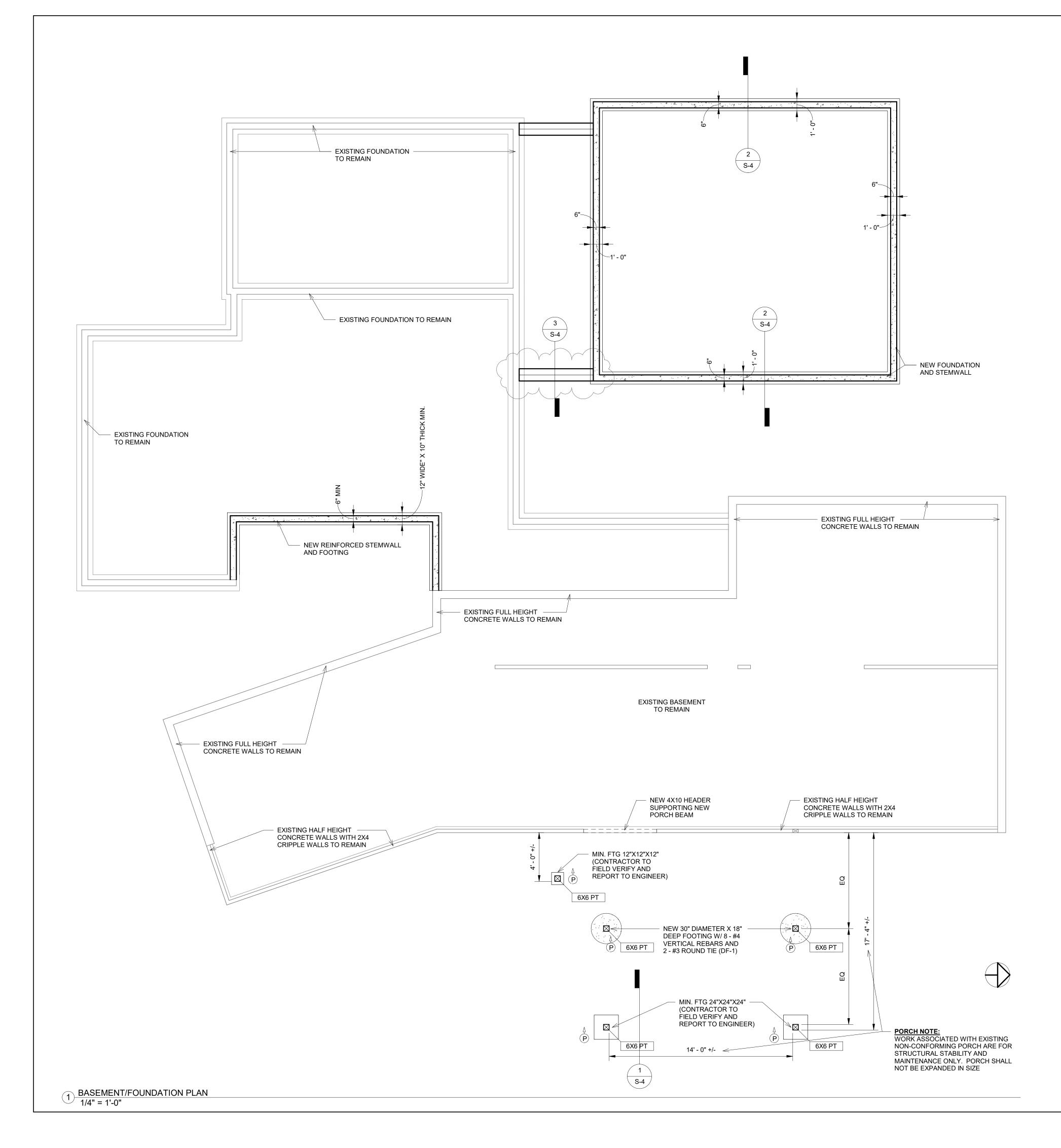
ASTM A307 (WOOD FRAMING) ASTM A325 (STEEL FRAMING)

ALL SLIP CRITICAL CONNECTIONS SHALL BE ASTM A325 BOLTS AND SHALL BE ENGINEER-APPROVED, SELF-LOAD INDICATING TYPES, AND SHALL BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

STRUCTURAL STEEL WELDING CONFORM TO THE AWS CODES D1.1 AND D1.3, AND USE ONLY CERTIFIED WELDERS. WELDS NOT SPECIFIED ARE TO BE 1/4" CONTINUOUS FILLET MINIMUM. INCREASE WELD SIZE TO AWS MINIMUM SIZES, BASED ON PLATE THICKNESS. USE DRY E70 ELECTRODES WELDING SHALL CONFORM TO THE AWS CODES, AND SHALL BE BY CERTIFIED WELDERS. WELDS NOT SPECIFIED SHALL BE 1/4" CONTINUOUS FILLET MINIMUM. USE DRY E70 ELECTRODES.

	E	XTENT		b2 structural engineers
	P	eriodic		info@b2engineers.com
		eriodic		425-318-7047 (O)
0 SF during construction	Р	eriodic		425-318-0031 (C)
ortar, prestressing grout, a e		continuous		
acclidating grout on dolive		eriodic continuous		
nsolidating grout as delive		eriodic		
for bonded tendons prestressing tendons and	Р	eriodic eriodic		
ndons and anchorages	L: L: L:	'eriodic evel B - Periodic evel C - Continuous evel B - Periodic evel C - Continuous		
5	С	continuous eriodic		
rage of masonry to struc	ural Lo	evel B - Periodic evel C - Continuous		
old weather (temperature	C	continuous reriodic		SRI BAR
		Continuous		A LE OF WIGHING P
d mortar joints (first 5000	SF of C	continuous		
d mortar joints (after the t	Le	evel B - Periodic evel C - Continuous		B Pr 43789
⁼ of AAC masonry) 5000 SF of AAC masonry	') La La La La La	continuous evel B - Periodic evel C - Continuous evel B - Periodic evel C - Continuous evel B - Periodic evel C - Continuous		B AUCTURAL ENGINE
assemblies in accordar	ce with P	eriodic		
nel sheathing agree with	Р	eriodic		
t adjoining panel edges, between fasteners in eac		eriodic		
ify temporary and perma		eriodic		
bmittal package				2906 74TH AVE
the design bearing capac		eriodic		
roper material. lacement and compactio	Р	eriodic eriodic eriodic		SE REMODEL
site has been prepared	C	continuous		
ene nas been prepared		eriodic		
s sts, as required ords for each element hammer, record number ign capacity, record tip al	C of blows C	continuous continuous continuous continuous		
2		0 1 1705 0		2906 74TH AVE SE, MERCER ISLAND, WA
nal inspections per Section	S	ee Section 1705.2 ee Section 1705.3		98040
I by the registered design	Ir	accordance with construction documents		90040
truction documents	Ir	accordance with construction documents		
rds for each element rs, bell diameters (if appl ring strata capacity. Reco	oubio),	Continuous Continuous		
with Section 1705.3 struction documents		ee Section 1705.3 a accordance with construction documents		
pth, final installation torqu	e and	continuous		
truction documents		ontinuous		
e-resisting system nents within the main wi	ndforce- C	continuous eriodic		
n ce ce-resisting system ing of components withir	the P	eriodic		DRAWING INFO
	P	eriodic		ISSUE DATE 08-30-22
		eriodic		
nce		accordance with AISC 341		ISSUED FOR PERMIT
INCE				PROJECT NO.20242
resisting system	Iorce-	continuous		
	P	eriodic		
pections for Seismic				REVISION SCHEDULE
	1 The	eriodic		NO. DATE DESCRIPTION
ance tance e resisting system bonents within the seismic spections for Seismic ce-resisting system ening of components withi	force- C P	continuous leriodic		
		DRAWING LIST		
	SHEET NUMBER		ISSUE DATE	
		SHEET NAME GENERAL NOTES AND	ISSUE DATE 08-30-22	GENERAL NOTES
	NUMBER S-0	SHEET NAME GENERAL NOTES AND SPECIFICATIONS	08-30-22	GENERAL NOTES
	NUMBER	SHEET NAME GENERAL NOTES AND		AND
ECTRODES. ALL	NUMBER S-0 S-1	SHEET NAME GENERAL NOTES AND SPECIFICATIONS BASEMENT/ FOUNDATION PLAN	08-30-22 08-30-22	

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	2906 74TH AVE SE, MERCER ISLAND, WA 98040
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E LS). TONS. NTS TED PTH) T EDGES	DRAWING INFO ISSUE DATE 08-30-22 ISSUED FOR PERMIT PROJECT NO.20242 ENGINEER BB REVISION SCHEDULE NO. DATE DESCRIPTION 2 08/30/22 Revision 2
GE BY CTION	BASEMENT/ FOUNDATION PLAN S-1 Copyright b2 Structural Engineers 2008

IMPORTANT NOTES ON FIELD VERIFICATIONS AND TEMPORARY SHORING:

1. CONTRACTOR SHALL REVIEW STRUCTURAL DRAWINGS AND FIELD VERIFY ALL RELATED EXISTING FRAMING & DIMENSIONS PRIOR TO ANY FIELD WORK. NOTIFY THE ENGINEER/OWNER ANY DISCREPANCIES FOUND IN THE FIELD. STRUCTURAL DRAWINGS MAY NOT CORRECTLY REFLECT ALL EXISTING FRAMING DUE TO LIMITED ACCESS TO THE SITE AND EXISTING DRAWINGS.

2. CONTRACTOR SHALL FIELD VERIFY AND NOTIFY THE ENGINEER/OWNER OF EXISTING MECHANICAL DUCTS, PLUMBING PIPES, ELECTRICAL WIRES THAT MAY INTERFERE WITH STRUCTURAL WORKS FOR COST CONSIDERATIONS PRIOR TO ANY FIELD WORK. 3. CONTRACTOR IS SOLELY RESPONSIBLE IN PROVIDING PROPER TEMPORARY SHORING PRIOR TO REMOVING ANY STRUCTURAL ELEMENTS. PLEASE CALL ENGINEER FOR QUESTIONS

IMPORTANT NOTES ON FOUNDATION AND FRAMING:

 ALL FOOTINGS SHALL BEAR ON SUITABLE SOIL SUCH AS MIN. OF MEDIUM DENSE NATIVE SOIL OR COMPACTED STRUCTURAL FILL (NO SOFT OR ORGANIC MATERIALS). GEOTECHNICAL ENGINEER MAY BE REQUIRED TO ASSESS EXISTING SOIL CONDITIONS
 FOR FRAMING LUMBER TYPES AND GRADES, AND CONCRETE MIX REQUIREMENTS PLEASE SEE S-0

3. FOR PLYWOOD/OSB SHEARWALL SCHEDULE, PLEASE SEE S-4

4. FOR COMMON HEADER FRAMING DETAIL AND HEADER SIZE, SEE S-4
5. PROVIDE (2) 2X6 OR (3) 2X4 STUD POSTS AT EACH END OF BEAMS, UNLESS NOTED OTHERWISE ON PLAN

6. SLAB ON GRADE SHALL BE MIN. 4" THICK WITH #3 AT 18" EACH WAY (AT MID-DEPTH) ON 6" COMPACTED CRUSHED ROCK. PROVIDE 1" SAWCUT JOINT AT 15 FT MAX. SPACING (EACH WAY)

7. FLOOR SHEATHING SHALL BE 3/4" PLYWOOD OR OSB WITH 10d AT 6" NAILING AT EDGES AND AT 12" AT FIELD
8. ROOF SHEATHING SHALL BE 1/2" PLYWOOD OR OSB WITH 8d AT 6" NAILING AT EDGES AND AT 8" AT FIELD

IMPORTANT NOTES ON TRUSS AND FLOOR FRAMING DESIGN/SHOP DRAWINGS:

1. TRUSS FRAMING LAYOUT SHOWN IS GENERAL CONCEPT ONLY. CONTRACTOR/

TRUSS SUPPLIER MUST SUBMIT TRUSS SHOP DRAWINGS INCLUDING TRUSS TEMPORARY/ PERMANENT BRACING PLANS FOR ENGINEER'S REVIEW

2. TRUSS FRAMING PROFILE/ LAYOUT SHOULD CONFORM TO BOTH STRUCTURAL AND ARCHITECTURAL DRAWINGS. ANY DEVIATIONS SHALL BE APPROVED BY ENGINEER/ ARCHITECT PRIOR TO TRUSS DESIGN WORK.

3. TRUSS DEFLECTION CRITERIAS:

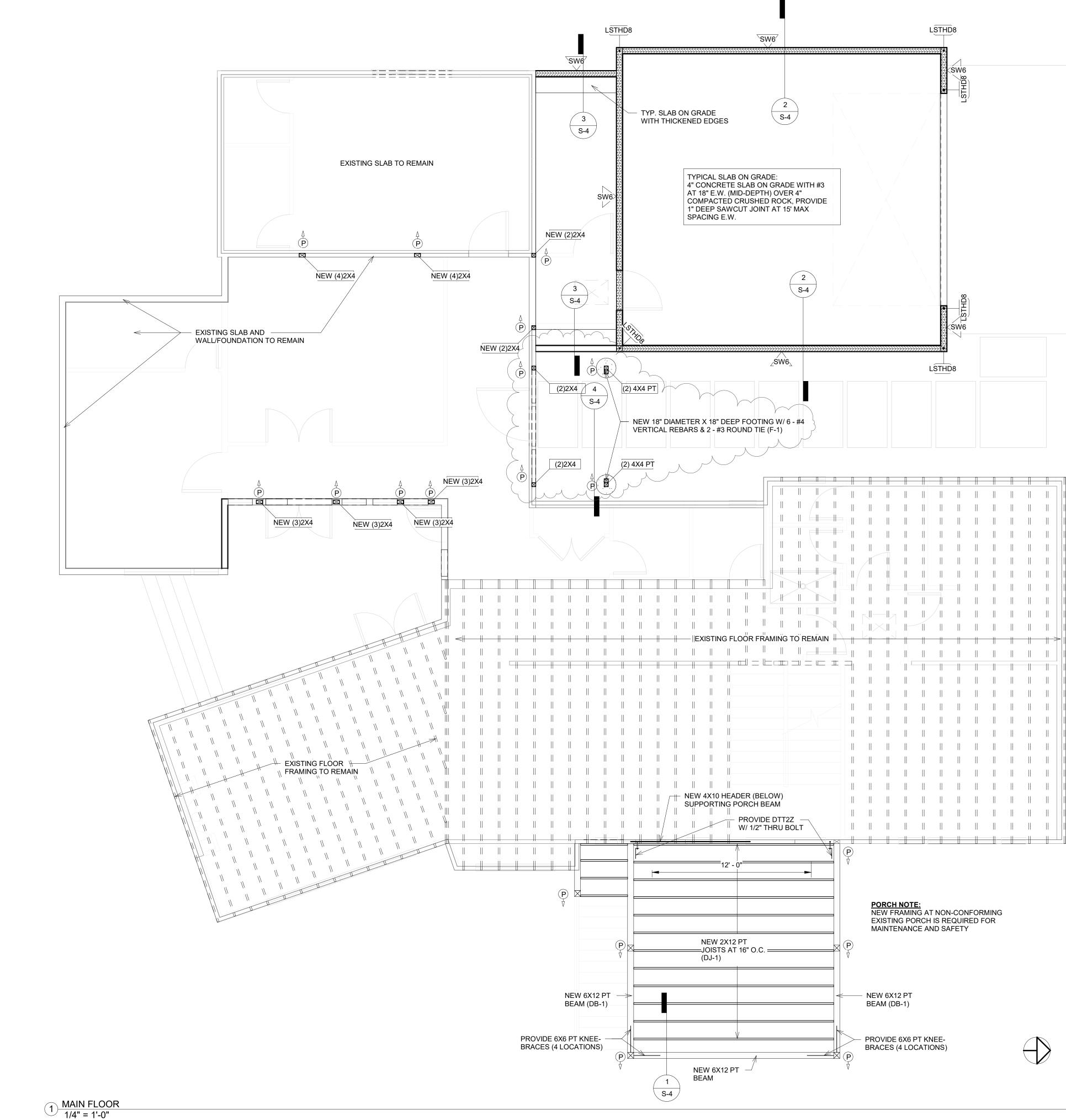
FLOOR/DECK TOTAL LOAD = L/480 FLOOR/DECK LIVE LOAD = L/600 ROOF TOTAL LOAD = L/240 ROOF SNOW LOAD = L/300

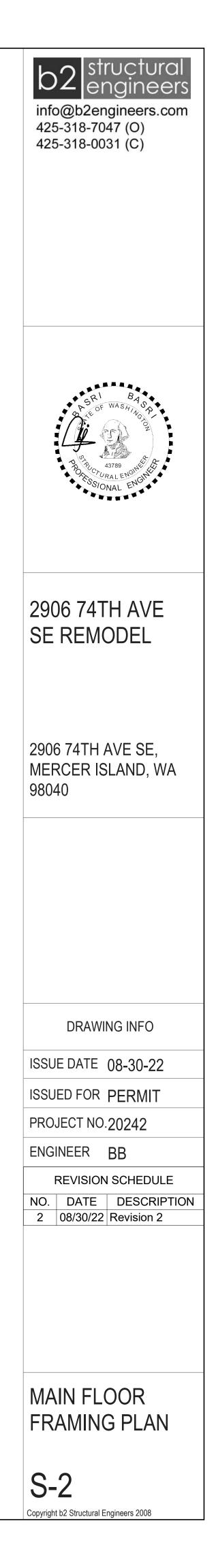
** MAXIMUM TOTAL LOAD DEFLECTION SHOULD NOT EXCEED 1.0" IN ALL CASES 4. FLOOR/ROOF FRAMING LAYOUT AND CONNECTORS (SUCH AS LUMBER PACKAGE BY SUPPLIERS) MUST BE SUBMITTED FOR ENGINEER'S REVIEW PRIOR TO CONSTRUCTION

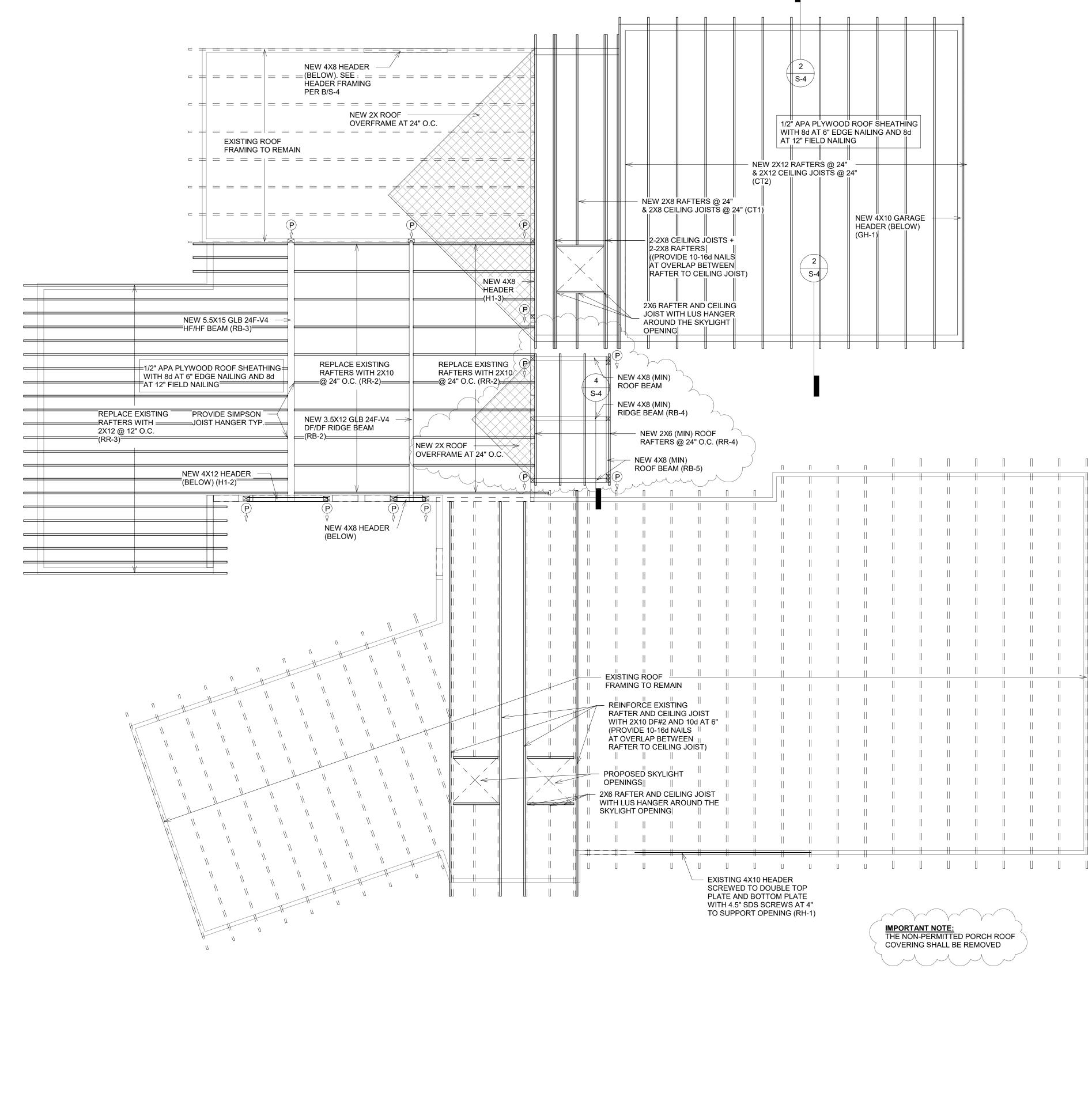
FRAMING SYMBOLS:

	011110000 <u>.</u>	٨	
SS24	SIMPSON WSW STRONG WALL (24" WIDE)	(P) ↓	CONTINOUS POST
SW6	PLYWOOD SHEARWAL	(P) ↓	POST STOPS BELOW THIS FLOOR
A	- SHEARWALL HOLDOWN	(P)	POST STARTS AT THIS FLOOR
	ND AND NOTES		

1/4" = 1'-0"

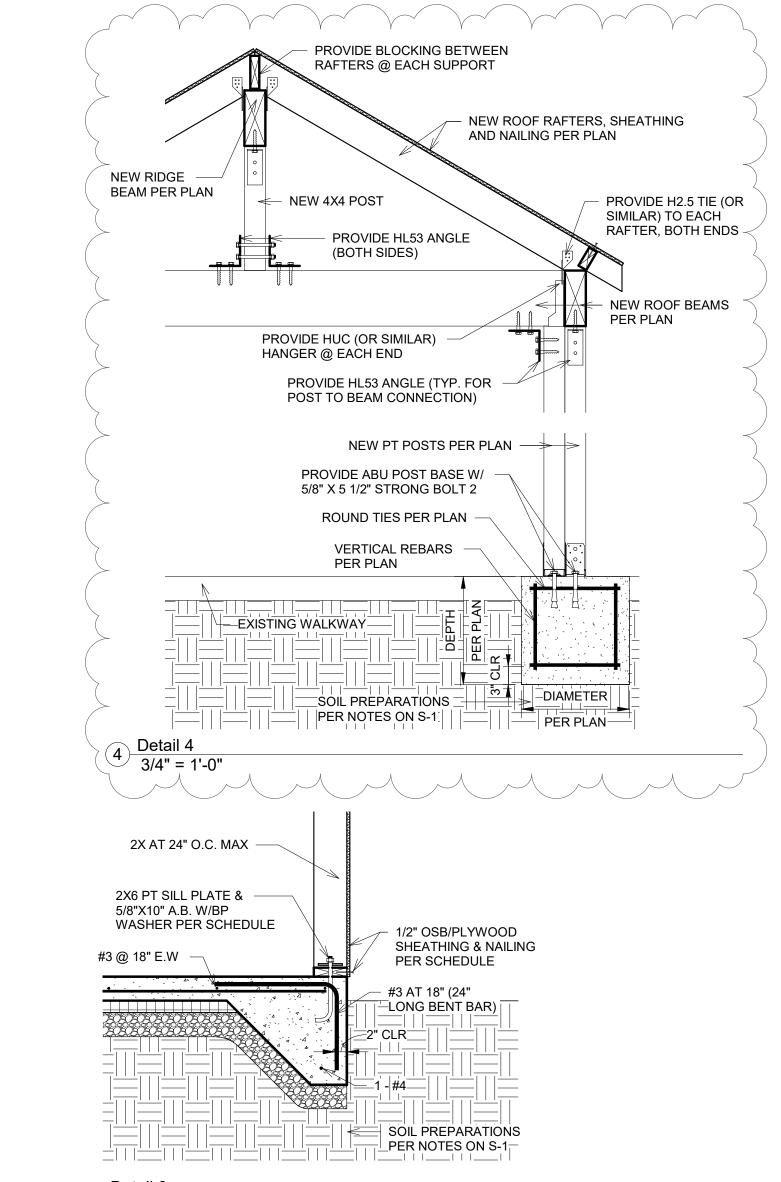




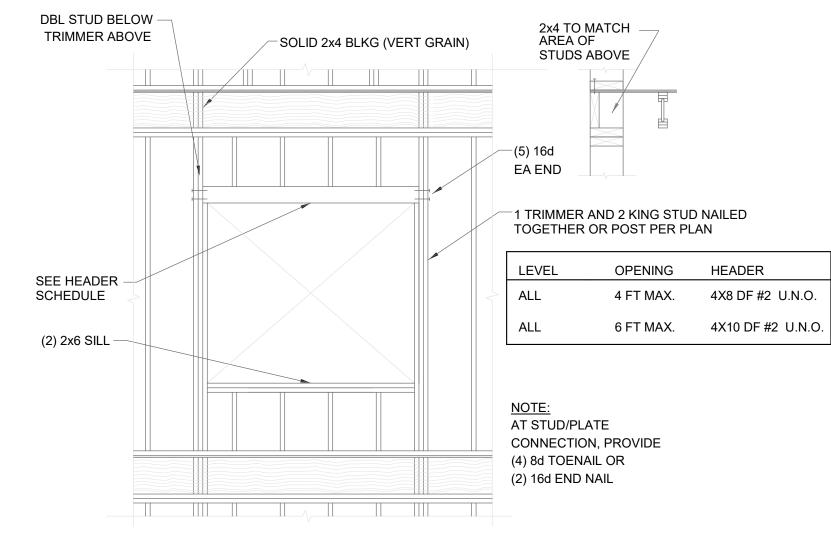


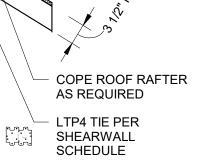
b2 structural engineers.com 425-318-7047 (O) 425-318-0031 (C)
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DRAWING INFO
ISSUE DATE 08-30-22 ISSUED FOR PERMIT
PROJECT NO.20242 ENGINEER BB
REVISION SCHEDULE
NO.DATEDESCRIPTION103/16/22Revision 1208/30/22Revision 2
ROOF FRAMING PLAN
S-3 Copyright b2 Structural Engineers 2008

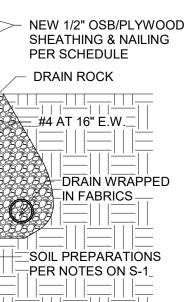
						SF	EW ROOF RAFTERS, IEATHING AND AILING PER PLAN
	۲ <u>۵</u>						/ PROVIDE (5) 3" LONG SDS
		AT 4' M RAILING			NEW CEILING JO PER PLAN		SCREWS BETWEEN CEILII JOIST/RAFTER (TYP.)
	•		NEW PT BEAMS PER			•	BIRD BLOCKING
	•		PLAN			,X	
	<i>▶</i>		JOISTS PER PLAN	R		PROVIDE H2.5 TIE (BOTH ENDS OF	
	•					ÈACH RAFTER)	
				_			
	DE (6) 6" LONG —		<u>u.</u>	- PROVIDE LUS			LTP4 TIE PER
	RLOK SCREWS			HANGER (BOTH EI	NDS)		
			Z	— PROVIDE KBS1Z K BRACE STABILIZEI		NEW 2X6 STUD	→ .
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						HOR BOLTS WITH BP	
	I				SLAB ON		NEW 1/2" OSB/PLYWOOD SHEATHING & NAILING
			(6 PT CONTINUOUS ER PLAN		PER PLA		PER SCHEDULE
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	·		/2" STRONG BOLT 2				•• #4 AT 16" E.W
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		2' - 0"	ਲ਼ <u></u> <u></u> Soil Preparations Per Notes on S-1_				
Detail 1 3/4" = 1'	·····				2 Detail 2 3/4" = 1'-0'	1	
TYPE	PLYWOOD OR OSB SHEATHING (NOTE 7)	PANEL EDGE NAILING (NOTE 4)	PANEL EDGE STUDS AND BLKG	ANCHOR BOLTS AT SILL PLATE (NOTE 8)		BOTTOM PLATE TO BLOCKING/ RIM (NOTE 4)	CAPACITY (LRFD) (SEISMIC/WIND)
SW6 1	SHEATHING (NOTE 7) 15/32" PLY/OSB ONE SIDE	NAILING (NOTE 4) 10d COM AT 6"	STUDS AND BLKG	SILL PLATE (NOTE 8) 5/8" AT 36" O.C2x	BLOCKING/ RIM (NOTE 9) SIMPSON LTP4 AT 24" O.C.	BLOCKING/ RIM (NOTE 4) 16d COM AT 6" O.CNARROW	(SEISMIC/ŴIND) 496 PLF/ 696 PLF
SW6 1 SW4 1	SHEATHING (NOTE 7) 15/32" PLY/OSB ONE SIDE 15/32" PLY/OSB ONE SIDE	NAILING (NOTE 4) 10d COM AT 6" 10d COM AT 4"	STUDS AND BLKG	SILL PLATE (NOTE 8) 5/8" AT 36" O.C2x 5/8" AT 24" O.C2x	BLOCKING/ RIM (NOTE 9)SIMPSON LTP4 AT 24" O.C.SIMPSON LTP4 AT 16" O.C.	BLOCKING/ RIM (NOTE 4)16d COM AT 6" O.CNARROW16d COM AT 4" O.CNARROW	(SEISMIC/WIND) 496 PLF/ 696 PLF 736 PLF/ 1032 PLF
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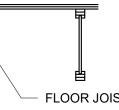


3 Detail 3 3/4" = 1'-0"









B TYP. WALL OPENING FRAMING 3/4" = 1'-0"

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